



**WORKSHOP 12-13 febbraio 2007**

*Materiali ed Approcci Innovativi per il Progetto in Zona Sismica e la Mitigazione della Vulnerabilità delle Strutture*

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# Progettazione sismica di edifici con controventi concentrici

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# Seismic design of CBFs

- **Design approach of recent seismic codes**
  - ❖ **capacity design concept for CBFs**
  - ❖ **ductile ultimate behaviour**
  
- **Design Provisions**
  - ❖ **slenderness of braces**
  - ❖ **(over)strength of beams, columns, connections**

*Influence of code provisions in the  
seismic design of ductile CBFs*

*plan – brace - building*

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**Examination of code provisions**

EC8 (march'94 and decemb.'03)

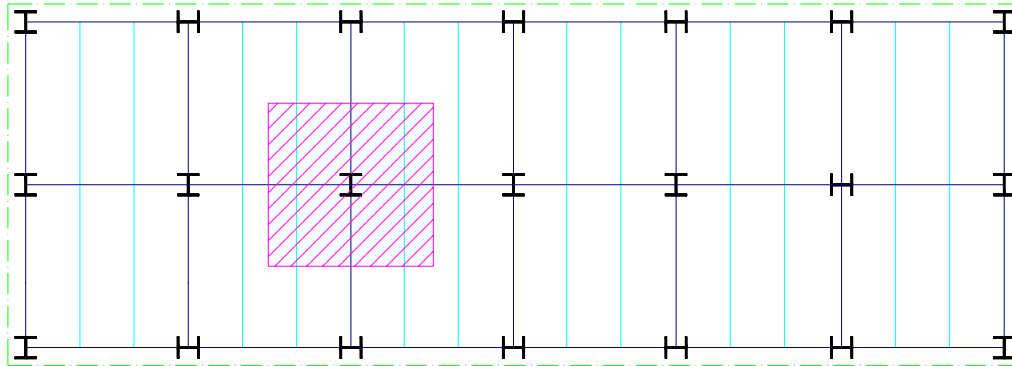
OPCM (march'03 and may'05)

**Design of CBF buildings:**


geometric plan-structure dimensions

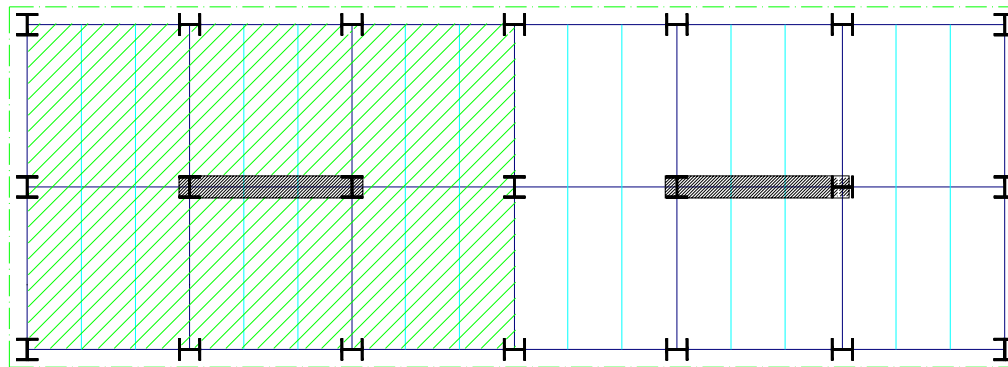
CBFs arrangement in plan

# Presented results



## Gravity Loads Resisting System

 Single column Tributary area for **gravity loads** resistance



## Seismic Loads Resisting System

 Braced Frames

 single brace tributary area for **seismic loads** resistance

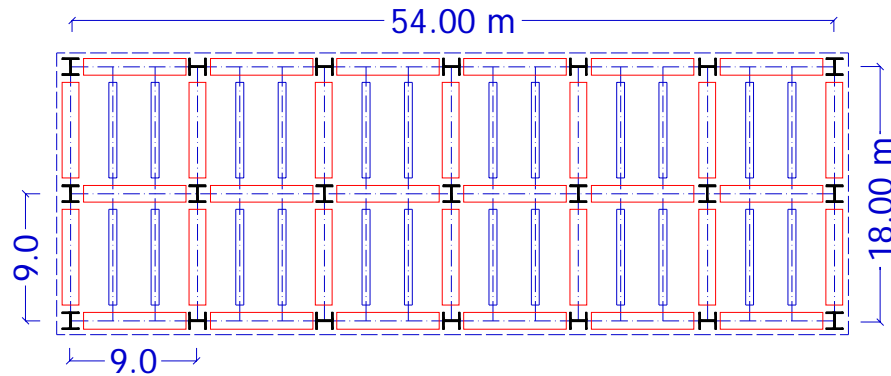
## 1. Design solutions:

$$Y = \text{n. of grid units} / \text{n. of braces} (12/2=6)$$

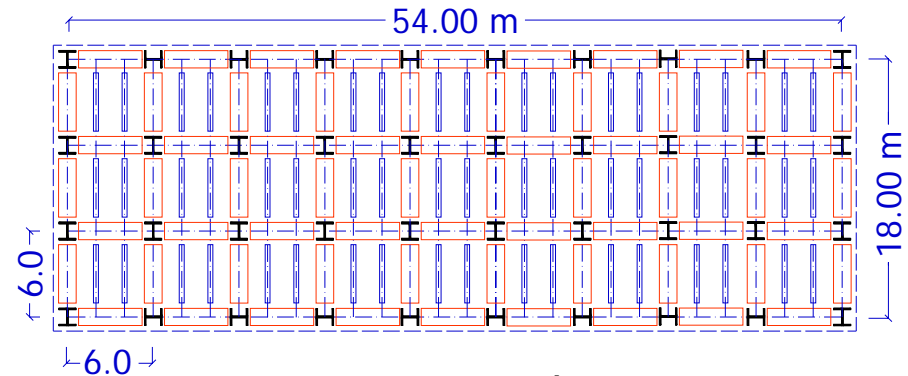
## 2. Structural weight:

$$\frac{P_{\text{CBF}}}{P_{\text{NF}}} = \frac{\text{Structural weight of gravity load resisting system}}{\text{Structural weight of CBF system}}$$

# Examined structures

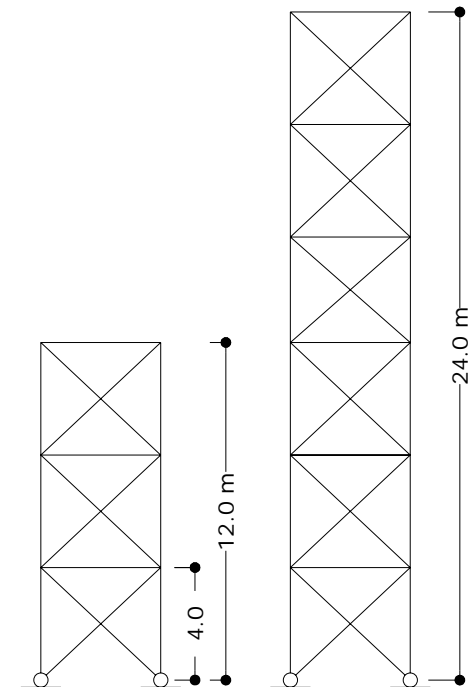


6 m span bays



9 m span bays

- ❑ building plan
- ❑ n. of stories: 3 and 6 story buildings
- ❑ seismic codes: EC8– OPCM  
DM96 and *elastic design*
- ❑ number of truss frames in plan
- ❑ max col. section : HE1000M (S355)



## 1) Slenderness of diagonal members

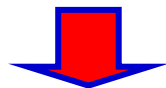
$\bar{\lambda} \leq \bar{\lambda}_{\max}$   min. size of diagonal sections

$\bar{\lambda} > \bar{\lambda}_{\min}$   max. size of diagonal sections

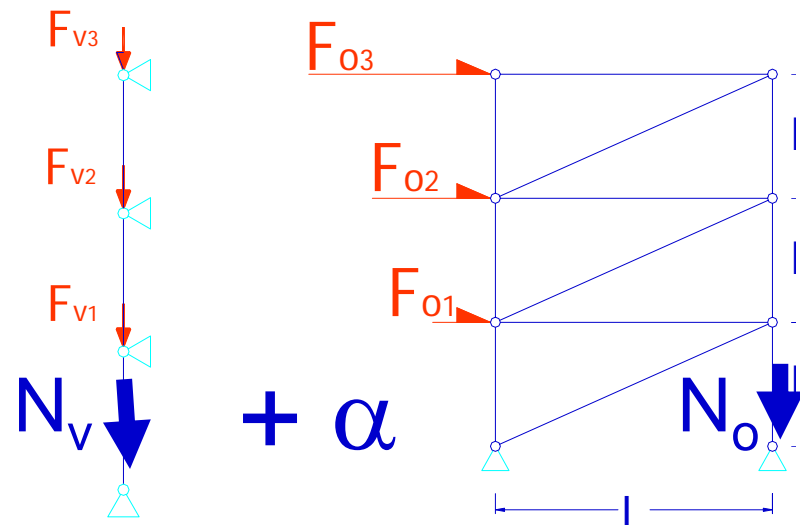
## 2) Strength of columns and beams

overstrength factor:  $\alpha_i = \left[ \frac{N_{pl,diag,i}}{N_{sd,diag,i}} \right]$

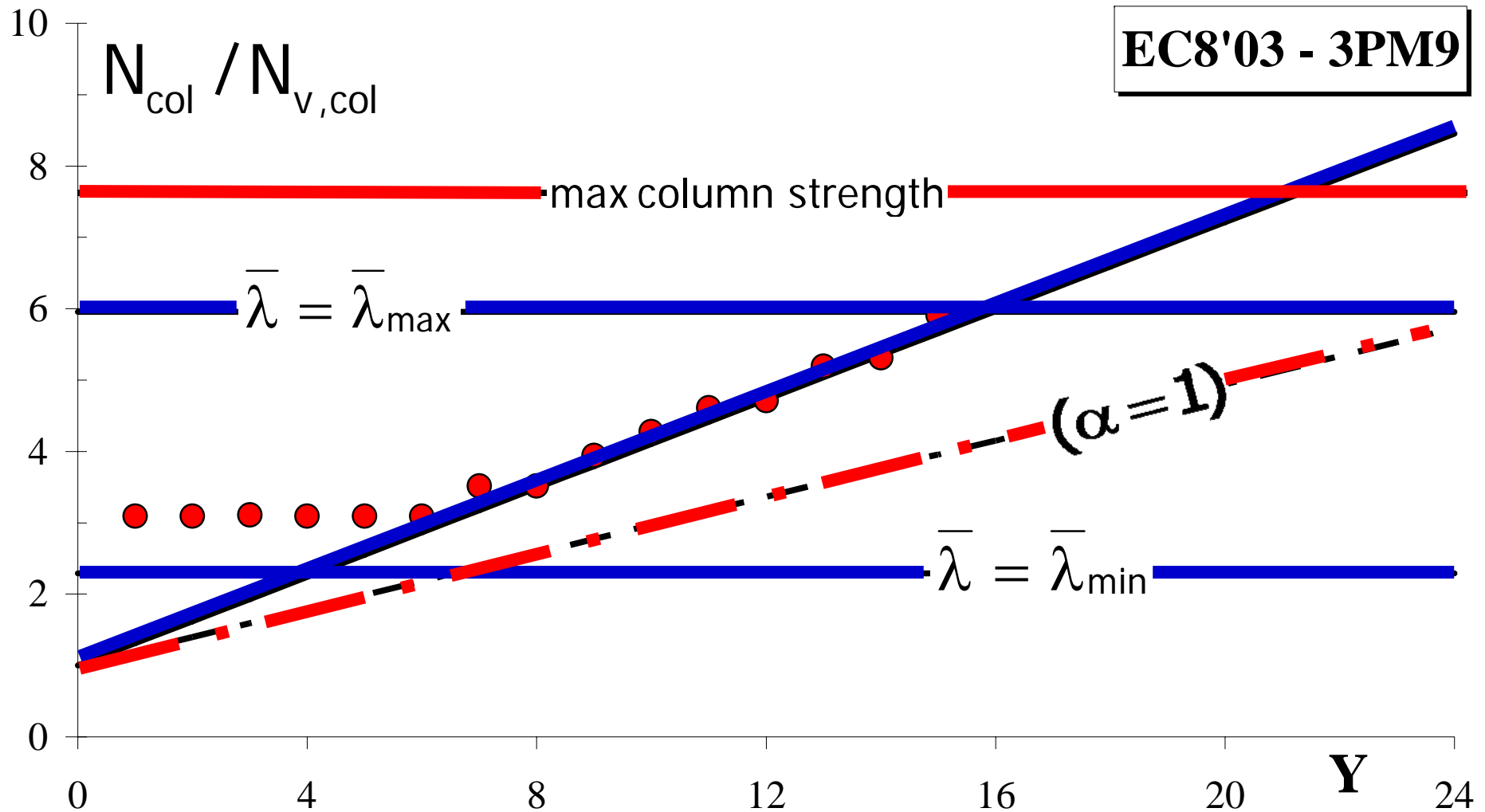
$$N_{col} = N_v + \alpha N_o$$



min. size of column sections

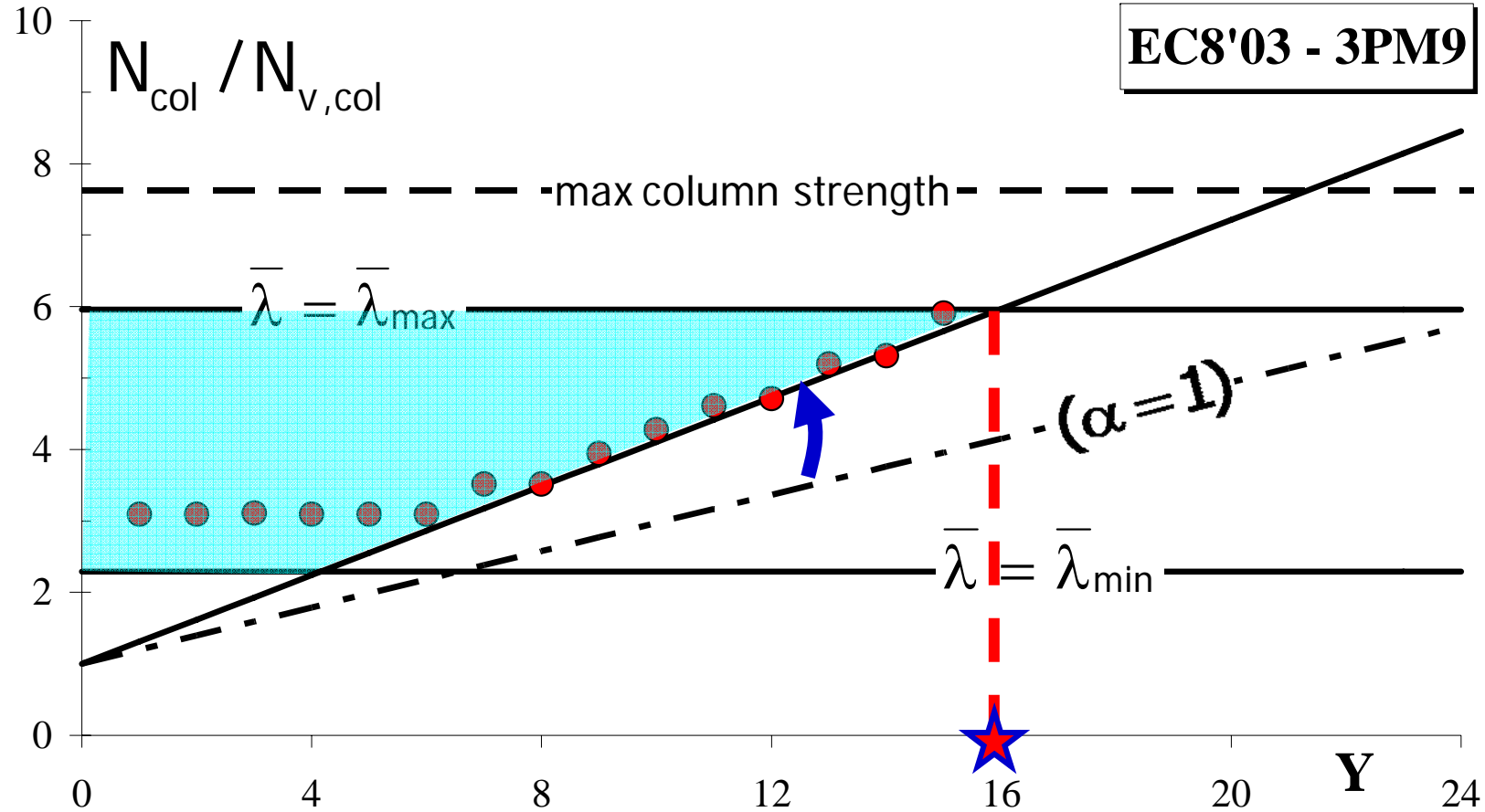


# Influence of Design Provisions



*column strength requirements*

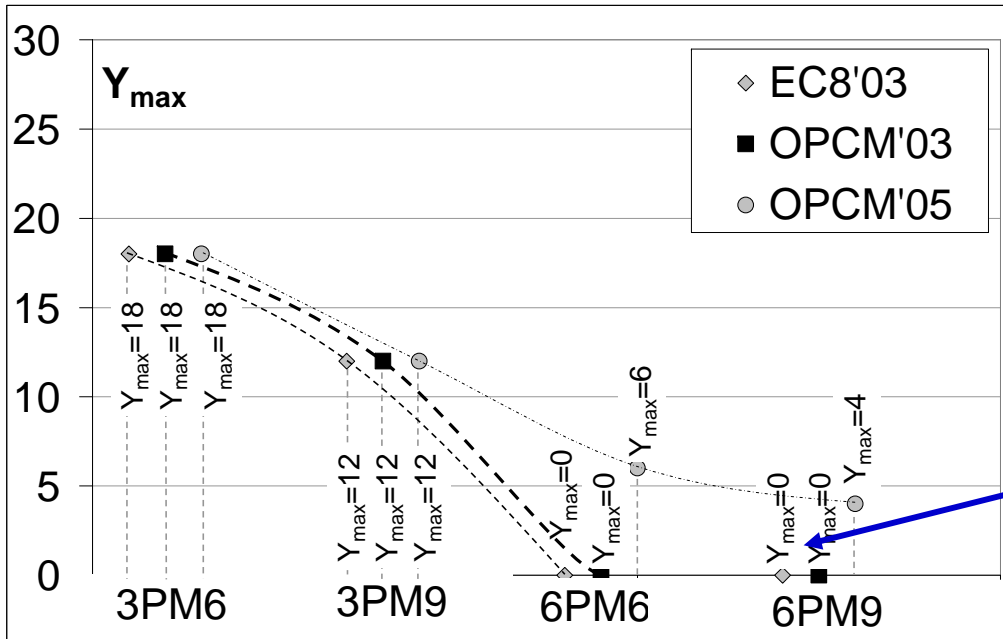
# Influence of Design Provisions



- ❖ Increasing of required column strengths
- ❖ Domain of admissible cases
- ❖ Design limits  $\Rightarrow Y_{max}$

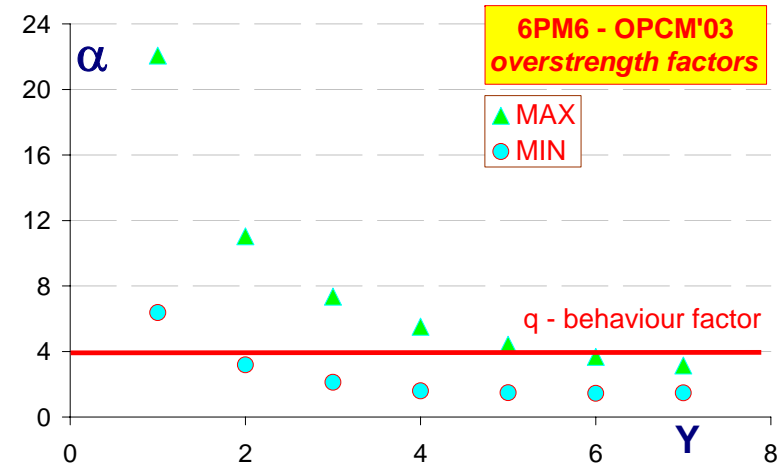
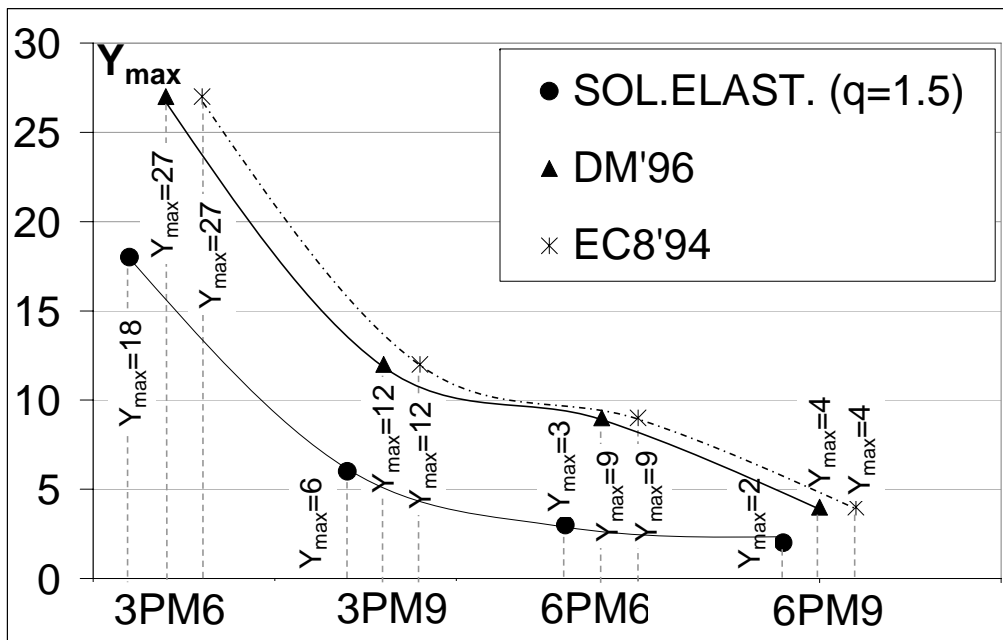
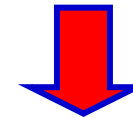


# Influence of Design Provisions

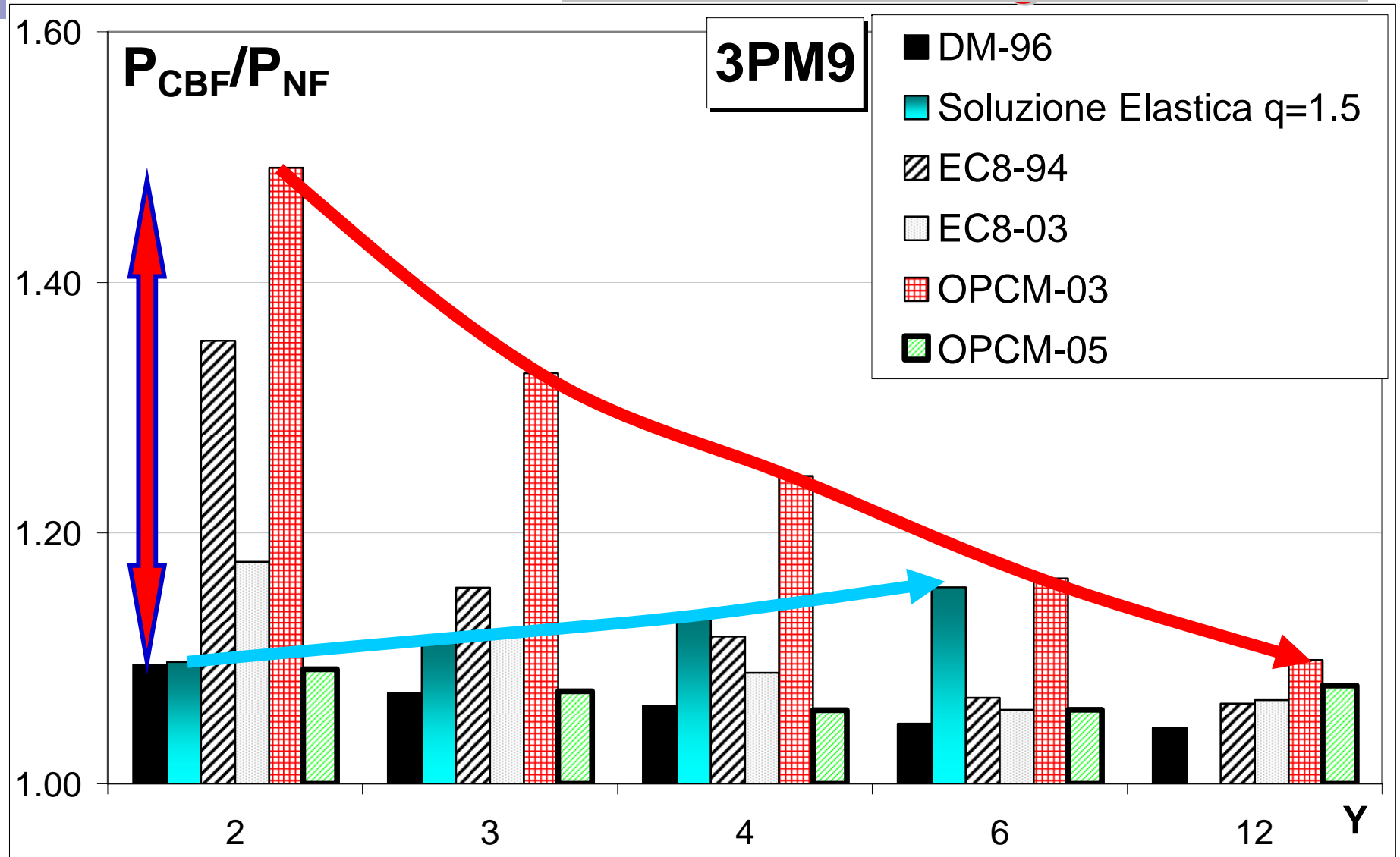


**Y<sub>max</sub> = max n. of grid units sustained by the single braced frame**

**6 story sol: no feasible sol. for EC8'03 and OPCM'03**



# Influence of Design Provisions



*structural weight*



## *CONCLUSIVE REMARKS*

- Geometric/structural parameters affecting X-CBF bldg design
  - domain of feasible structural solutions
- Limitations on diagonal slenderness and overstrength requirements for columns (and beams) strongly affects the dimensioning process of CBFs
- Weight premium due to capacity design can be high
- Elastic design of CBFs is more convenient in terms of structural weight when the diagonal stress level is low (high number of braces in plan) or when high overstrength factors are required